

GLUED LAMINATED BEAM DESIGN TABLES



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WOOD

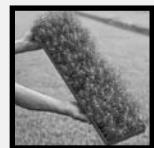
The Natural Choice



Engineered wood products are a good choice for the environment. They are manufactured for years of trouble-free, dependable use. They help reduce waste by decreasing disposal costs and product damage. Wood is a renewable, recyclable, biodegradable resource that is easily manufactured into a variety of viable products.

A few facts about wood.

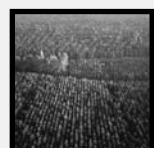
■ **We're growing more wood every day.** Forests fully cover one-third of the United States' and one-half of Canada's land mass. American landowners plant more than two billion trees every year. In addition, millions of trees seed naturally. The forest products industry, which comprises about 15 percent of forestland ownership, is responsible for 41 percent of replanted forest acreage. That works out to more than one billion trees a year, or about three million trees planted every day. This high rate of replanting accounts for the fact that each year, 27 percent more timber is grown than is harvested. Canada's replanting record shows a fourfold increase in the number of trees planted between 1975 and 1990.



■ **Life Cycle Assessment shows wood is the greenest building product.** A 2004 Consortium for Research on Renewable Industrial Materials (CORRIM) study gave scientific validation to the strength of wood as a green building product. In examining building products' life cycles – from extraction of the raw material to demolition of the building at the end of its long lifespan – CORRIM found that wood was better for the environment than steel or concrete in terms of embodied energy, global warming potential, air emissions, water emissions and solid waste production. For the complete details of the report, visit www.CORRIM.org.

■ **Manufacturing wood is energy efficient.** Wood products made up 47 percent of all industrial raw materials manufactured in the United States, yet consumed only 4 percent of the energy needed to manufacture all industrial raw materials, according to a 1987 study.

Material	Percent of Production	Percent of Energy Use
Wood	47	4
Steel	23	48
Aluminum	2	8



■ **Good news for a healthy planet.** For every ton of wood grown, a young forest produces 1.07 tons of oxygen and absorbs 1.47 tons of carbon dioxide.

Wood: It's the natural choice for the environment, for design and for strong, lasting construction.

GLUED LAMINATED BEAM DESIGN TABLES

Glued laminated beams (*glulams*) are used in a wide range of applications in both commercial and residential construction. The tables in this data file provide recommended preliminary design loads for two of the most common glulam beam applications: roofs and floors.

The recommendations in this publication apply to glulam beams bearing the APA EWS trademark. The mark appears only on beams manufactured by APA members and signifies that beams are produced to the requirements of American National Standards Institute (ANSI) Standard A190.1. This is the national consensus standard recognized by all model code agencies for the manufacture and trademarking of glulam.

The tables included in this data file include values for section properties and capacities, and allowable loads for simple span and cantilevered beams. The tables are based on an allowable bending stress of $F_b = 2,400$ psi for both Douglas-fir and southern pine.

These tables assume the compression edge of the beam is braced to prevent lateral buckling. For other bracing conditions, the beams should be checked for lateral stability.

For Douglas-fir, an allowable horizontal shear stress of $F_v = 265$ psi was used. For southern pine, an allowable horizontal shear stress of $F_v = 300$ psi was used.

Glulam is also an excellent choice for vertical load carrying members (i.e., posts or columns). For information on the use of glulam for these applications, see APA publication, *Design of Structural Glued Laminated Timber Columns*, Form Y240.

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Section Properties and Capacities

Tables 1 and 7 provide section properties and capacities for two commonly used species of glulam beams under dry-use conditions. Bending moment and shear capacities are based on a normal (10-year) duration of load. Dimensions shown are net sizes, and capacities are based on loading perpendicular to the wide faces of the laminations; that is, bending about the x-x axis of the beam, as shown in Figure 1. **Final design should include a complete analysis, including bearing stresses and lateral stability.**

See Design Examples 1 and 4 (pages 24 and 26) for examples of preliminary design using glulam beam section capacities from Tables 1 and 7.

Allowable Loads for Simple Span Glulam Beams

Tables 2, 3, 8 and 9 provide allowable loads for glulam beams used as simple span roof members in snow load areas (DOL factor = 1.15) and for non-snow loads (DOL factor = 1.25). Tables 4 and 10 provide similar information for floor members. The tables can be used to size such members for preliminary design. **Final design should include a complete analysis, including bearing stresses and lateral stability.**

See Design Examples 2 and 3 (pages 24-25) for examples of preliminary design using glulam beam load-span tables.

Allowable Loads for Cantilevered Glulam Roof Beams

Tables 5, 6, 11 and 12 are for preliminary design of cantilevered roof beams. The tables are based on balanced (fully loaded) as well as unbalanced loading. They do **not** include deflection criteria limitations. Final designs should include deflection requirements per the applicable building code, in addition to the bending and shear strength assessments incorporated in these tables. **Final design should include a complete analysis, including bearing stresses and lateral stability.**

A minimum roof slope of 1/4 inch per foot in addition to specified camber is recommended to help avoid ponding of water on the roof.

The cantilever beam tables presented are applicable to balanced layups, such as 24F-V8 for Douglas-fir and 24F-V5 for southern pine, for three different systems. See Figure 2 for details of the following typical cantilever systems:

- System 1 is a two-equal-span cantilever system with the cantilevered beam extending past the center support by approximately 0.20 times the span, or 0.20L. Its overall length is therefore 1.2L, and the suspended beam's length is 0.8L.
- System 2 is a three-equal-span cantilever system with each of the two outer cantilevered beams extending past the center support into the middle span by 0.25L. Their length is therefore 1.25L, and the interior suspended beam's length is 0.5L.
- System 3 is also a three-equal-span cantilever system, but the two outer span beams are suspended from the interior, double cantilevered beam, which extends past its two supports by approximately 0.17L. Its length is 1.34L, and the suspended beams are 0.83L each.

FIGURE 1

TYPICAL GLULAM BEAM CROSS SECTION

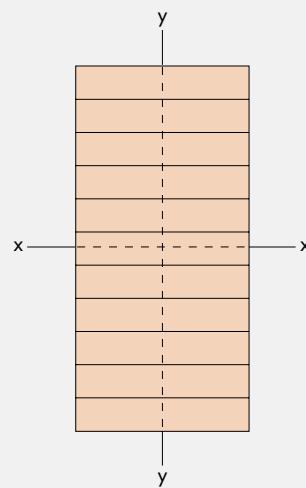
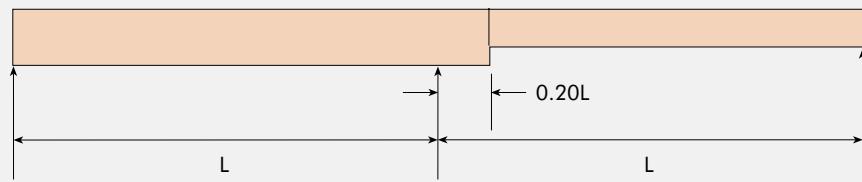


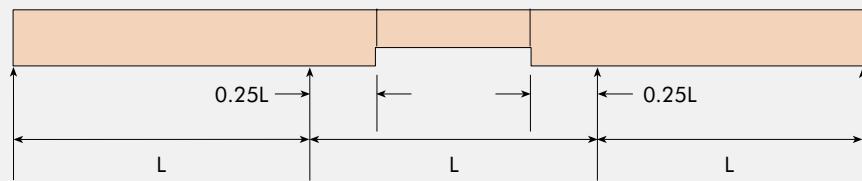
FIGURE 2

TYPICAL CANTILEVER BEAM SYSTEMS

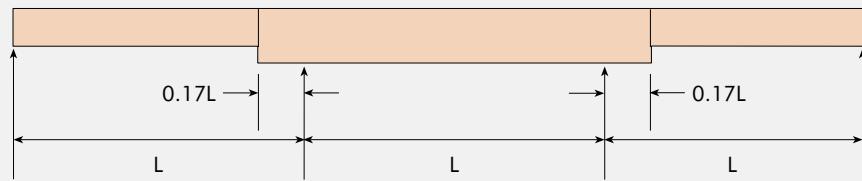
SYSTEM 1



SYSTEM 2



SYSTEM 3



DESIGN EXAMPLES**DESIGN EXAMPLE 1 – Low Slope Roof Design Using Section Capacities****Given:**

- 24-ft span, 24-ft-wide tributary area
- Live load = 30 psf (snow); Duration of load = 1.15
- Dead load = 10 psf (actual)
- Allowable total load deflection = L/180
Allowable live load deflection = L/240
- Use 24F Douglas-fir glulam

Then:

- Glulam span = 24 ft
- Load, $w = (30 + 10)(24) = 960 \text{ lb/ft}$ to glulam
- Max. Moment = $\frac{wL^2}{8} = 960 \times \frac{24^2}{8} = 69,120 \text{ lb-ft}$
- Max. Shear = $\frac{wL}{2} = 960 \times \frac{24}{2} = 11,520 \text{ lb}$

Design:

- From Table 1, try 5-1/8 x 21 (weight = 26 lb/ft)
Total load = $960 + 26 = 986 \text{ lb/ft}$
- From Appendix A, volume factor = 0.9330
- Design moment capacity = $75,340 \times 0.9330 \times 1.15 = 80,836 \text{ lb-ft}$
 $69,120 \times \frac{986}{960} = 70,992 \text{ lb-ft} < 80,836 \text{ lb-ft}$ OK
- Design shear capacity = $19,010 \times 1.15 = 21,862 \text{ lb}$
(For shear design, neglect all loads within a distance from supports equal to the depth of the beam)
 $11,520 \times \frac{986}{960} - \left(\frac{21}{12} \times 986 \right) = 10,107 \text{ lb} < 21,862 \text{ lb}$ OK
- Deflection, total load = $\frac{5wL^4}{384 EI} = \frac{5 \times 986 \times 24^4 \times 1,728}{384 \times 7,119 \times 10^6} = 1.03 \text{ in.} = L/279 < L/180$ OK
- Deflection, live load = $\frac{30 \times 24}{986} \times 1.03 = 0.75 \text{ in.} = L/383 < L/240$ OK

DESIGN EXAMPLE 2 – Low Slope Roof Design Using Load-Span Tables**Given:**

- 24-ft span, 24-ft-wide tributary area
- Live load = 30 psf (snow); Duration of load = 1.15
- Dead load = 10 psf (actual)
- Maximum deflection under total load = L/180
- Use 24F southern pine glulam

Then:

- Total applied load, $w = (30 + 10)(24) = 960 \text{ lb/ft}$, excluding beam weight
- From Table 9 for 24-ft span, select 3 x 26-1/8 ($w = 1,050 \text{ lb/ft}$)
 - or 3-1/2 x 23-3/8 ($w = 977 \text{ lb/ft}$)
 - or 5 x 20-5/8 ($w = 1,070 \text{ lb/ft}$)
 - or 5-1/2 x 19-1/4 ($w = 1,023 \text{ lb/ft}$)
 - or 6-3/4 x 17-7/8 ($w = 1,073 \text{ lb/ft}$)

Note that the beam weight is included in the table.

DESIGN EXAMPLE 3 – Panelized Roof Design Using Load-Span Tables

A warehouse/office building is to be 85 ft x 180 ft. It has a “flat” roof with a minimum slope of 1/4:12. The design live load (non-snow load) is the minimum required by the International Building Code (IBC), with a duration of load factor of 1.25. Assume design dead load = 8 psf. It is desired to minimize the number of interior columns.

Assume three 60-ft bays (equals 180 ft) and two 42.5-ft bays (equals 85 ft) requiring two interior columns.

Main Beam Design

Option 1

Try System 3 (double cantilever) with three 60-ft bays. The tributary area for each cantilever beam's main span is $60 \times 42.5 = 2,550 \text{ ft}^2$. The suspended beam's tributary area is $0.83 \times 60 \times 42.5 = 2,117 \text{ ft}^2$. Per Section 1607.11.2.1 of the 2006 IBC, the minimum design live load is 12 psf for tributary areas greater than 600 ft^2 per beam. Therefore, the design live load for these beams is $12 \times 42.5 = 510 \text{ lb/ft}$ and the design total load, excluding beam weight, is $(12 + 8) \times 42.5 = 850 \text{ lb/ft}$.

Assume 24F-V8 Douglas-fir glulam with $F_b = 2,400 \text{ psi}$ and $E = 1,800,000 \text{ psi}$ for the main cantilever beam. From Table 5, a double cantilever beam (System 3) with 60-ft span, 6-3/4-inches wide and 37-1/2-inches deep can carry 897 lb/ft. Note that the beam weight has been included in the table. OK.

From Table 2, a simple span 24F-V4 Douglas-fir glulam beam 50 ft (0.83×60) long, 6-3/4-inches wide and 36-inches deep can carry 873 lb/ft. Note that the beam weight has been included in the table. OK.

Option 2

Try System 2 (single cantilever with suspended center beam) with three 60-ft bays.

Loads are the same as for Option 1, since all members carry more than 600 ft^2 of tributary area.

From Table 5, a single cantilever beam (System 2) with a 60-ft main span, 6-3/4-inches wide and 40-1/2-inches deep can carry 870 lb/ft. Note that the beam weight has been included in the table. OK.

From Table 2, a simple span beam 30 ft ($2 \times 0.25 \times 60$) long, 5-1/8-inches wide and 24-inches deep can carry 954 lb/ft. Note that the beam weight is included in the table. OK.

Note: A 6-3/4- x 21-inch beam can carry 944 lb/ft, which is acceptable, but its area of 142 in.² is greater than the area of the 5-1/8- x 24-inch beam (123 in.²), suggesting it may be less economical.

The two options can then be compared by beam volume, with the smaller volume typically indicating the most economical option.

Beam Volume for Option 1

$$\frac{6.75 \times 37.5}{144} (1 + 2 \times 0.17) 60 + \frac{2(6.75 \times 36)}{144} (0.83 \times 60) = 309.4 \text{ ft}^3$$

Beam Volume for Option 2

$$\frac{2(6.75 \times 40.5)}{144} (1 + 0.25) 60 + \frac{(5.125 \times 24)}{144} [(1 - 2 \times 0.25) \times 60] = 310.4 \text{ ft}^3$$

For this example, the beam volumes are approximately equal and the final selection is the designer's option.

Secondary Beam Design

Secondary beams, all perpendicular to the main beams and all simple span, are spaced at 8 ft on center as is typical with a panelized system panel deck. For a non-panelized system, they could be spaced farther apart, such as 20 ft on center, with subpurpils between these members at a closer on center spacing.

The secondary beams have a simple span of approximately 42 ft.

Assume secondary beams 8 ft on center. The tributary area is $42 \times 8 = 336 \text{ ft}^2$. Per Section 1607.11.2.1 of the 2006 IBC, the design live load is 18 psf. Total load, excluding beam weight, is $8(18 + 8) = 208 \text{ lb/ft}$. From Table 2, a simple span beam 42-ft long, 3-1/8-inches wide and 24-inches deep can carry 241 lb/ft. OK.

Other types of framing members, such as solid-sawn lumber, wood I-joists or wood trusses can also be used as secondary beams depending on the span and loading conditions.

A comparison of material costs will provide guidance as to their relative economies. In addition, hardware (hanger) requirements, as well as any labor differences, need to be considered in order to obtain a complete economic comparison of the systems.

DESIGN EXAMPLE 4 – Floor Design Using Section Capacities

Given:

- Two-span continuous beams with spans of 23.25 ft and 19.25 ft. Beams spaced at 10 ft on center.
- Floor live load = 125 psf (light storage); Duration of load = 1.0
- Dead load = 10 psf (actual)
- Allowable total load deflection = $L/240$
- Allowable live load deflection = $L/360$
- Beam depth limited to 24 inches or less, due to height restrictions
- Use 24F-V5 southern pine glulam

Then:

- Assume beam weight of 36 lb/ft
- Live load, $w_e = 125 \times 10 = 1,250 \text{ lb/ft}$
- Dead load, $w_d = (10 \times 10) + 36 = 136 \text{ lb/ft}$
- Total load, $w_t = 1250 + 136 = 1,386 \text{ lb/ft}$
- Maximum moment, fully loaded, $M = 80,312 \text{ lb/ft}$, at interior reaction
- Maximum moment, unbalanced loading, $M_u = 69,790 \text{ lb/ft}$ at approximately 10 ft from the outer support of the 23.25-ft span
- Maximum shear, fully loaded, $V = 16,795 \text{ lb}$ at 24 inches away from the interior reaction, in the 23.25-ft span
- Maximum shear, unbalanced loading, $V_u = 15,544 \text{ lb}$
- Maximum reaction, $R = 37,079 \text{ lb}$ at interior support

Design:

- From Table 7, a 3-1/2-inch-wide beam would exceed the depth limitation, based on shear requirements.
- Try a 5-inch-wide x 23-3/8-inch-deep beam. (For purposes of the volume factor, the moment capacity span is the distance between points of zero moment and is approximately 20 ft.) From Table 7 and Appendix B, the allowable moment capacity = $91,070 \times 0.9708 = 88,411 \text{ lb-ft} > 80,312 \text{ lb-ft}$. The actual beam weight of 29.2 lb/ft is less than the assumed 36 lb/ft. OK.
- The allowable compression perpendicular to grain, $F_{c\perp} = 740 \text{ psi}$. Minimum bearing length at interior support = $\frac{37,079}{740 \times 5} = 10 \text{ inches}$. Revised design shear, $V = 16,867 \text{ lb}$ at 23-3/8 inches away from the face of the interior support $< 21,038 \text{ lb}$. OK.
- Maximum deflection: total load on longer span, dead load only on shorter span = 0.66 in. = $L/425 < L/240$. OK.
- Maximum deflection: live load on longer span = 0.62 in. = $L/454 < L/360$. OK.

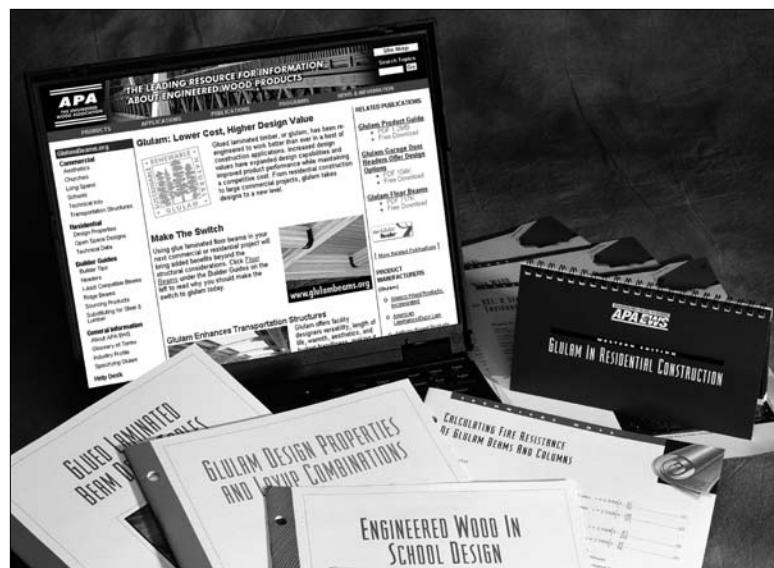
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HEADQUARTERS

7011 So. 19th St. • Tacoma, Washington 98466 • (253) 565-6600 • Fax: (253) 565-7265

www.apawood.org



PRODUCT SUPPORT HELP DESK

(253) 620-7400 • E-mail Address: help@apawood.org

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